



# MACHAKOS UNIVERSITY

DEPARTMENT OF BUILDING AND CIVIL ENGINEERING

DEGREE OF BACHELOR OF SCIENCE

YEAR 3 SEMESTER II EXAMINATIONS MAY 2019

ECV 412: DESIGN II

## Instructions to candidates

- This paper consists of **FIVE** questions.
- All symbols have their usual meaning unless otherwise stated
- Only relevant design codes are allowed
- Attempt question one and any other two questions

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## QUESTION ONE (30 Marks)

- List four types of foundations that may be used in construction of reinforced concrete structures. **(4 Marks)**
- Clearly describe two ways that may be used to achieve pre-stressing in concrete construction. **(8 Marks)**
- A solid footing has to transfer a dead load of 1000 kN and an imposed load of 400 kN from a square column 400 x 400 mm (with 16 mm bars). Assuming  $f_y = 415$ , and  $f_{ck} = 20$  N/mm<sup>2</sup>, and safe bearing capacity to be 200 kN/m<sup>2</sup>, design the footing. **(18 Marks)**

## QUESTION TWO (20 Marks)

- State five serviceability limit states that may be considered during design. **(5 Marks)**
- Design the longitudinal and shear reinforcement for a 275 mm square, short-braced column which supports an ultimate axial load of 1280 kN and bending moments of 35 kNm about the x-x axis and 25 kNm about the y-y axis. Assume  $f_{cu} = 30$  N/mm<sup>2</sup> and  $f_y = 460$  N/mm<sup>2</sup> and cover to all reinforcement is 35 mm. **(15 Marks)**

## QUESTION THREE (20 Marks)

Design a reinforced concrete slab 6.3 x 4.5 m simply supported on all the four sides. It has to carry a characteristic live load of 10 kN/m<sup>2</sup> in addition to its dead weight. Assume C25 concrete and  $f_y = 415$  N/mm<sup>2</sup>; also assume that the exposure condition to environment can be classified as mild. **(20 Marks)**

## QUESTION FOUR (20 Marks)

- (i) Design the longitudinal steel and links for a 300 mm square reinforced concrete column, short-braced which is to carry an ultimate load of 1400 kN, with the following material properties;  $f_{cu} = 25 \text{ N/mm}^2$  and  $f_y = 460 \text{ N/mm}^2$ . **(14 Marks)**
- (ii) State any three ways in which a reinforced concrete column may fail under loading. **(6 Marks)**

**QUESTION FIVE (20 Marks)**

A reinforced concrete beam has an overall depth of 500 mm and width of 300 mm. The beam is simply supported on 200 mm wide walls and spans 6 m. The beam is exposed to mild conditions. The beam is to carry an imposed load of 25 kN/m and a dead load of 15 kN/m (including self-weight). Assume  $f_{cu} = 30 \text{ N/mm}^2$ ,  $f_y = 460 \text{ N/mm}^2$  and  $f_{yv} = 250 \text{ N/mm}^2$ , design the beam together with the shear reinforcement. **(20 Marks)**

**Table 3.10** Cross-sectional areas of groups of bars (mm<sup>2</sup>)

| Bar size<br>(mm) | Number of bars |      |      |      |      |      |      |       |       |       |
|------------------|----------------|------|------|------|------|------|------|-------|-------|-------|
|                  | 1              | 2    | 3    | 4    | 5    | 6    | 7    | 8     | 9     | 10    |
| 6                | 28.3           | 56.6 | 84.9 | 113  | 142  | 170  | 198  | 226   | 255   | 283   |
| 8                | 50.3           | 101  | 151  | 201  | 252  | 302  | 352  | 402   | 453   | 503   |
| 10               | 78.5           | 157  | 236  | 314  | 393  | 471  | 550  | 628   | 707   | 785   |
| 12               | 113            | 226  | 339  | 452  | 566  | 679  | 792  | 905   | 1020  | 1130  |
| 16               | 201            | 402  | 603  | 804  | 1010 | 1210 | 1410 | 1610  | 1810  | 2010  |
| 20               | 314            | 628  | 943  | 1260 | 1570 | 1890 | 2200 | 2510  | 2830  | 3140  |
| 25               | 491            | 982  | 1470 | 1960 | 2450 | 2950 | 3440 | 3930  | 4420  | 4910  |
| 32               | 804            | 1610 | 2410 | 3220 | 4020 | 4830 | 5630 | 6430  | 7240  | 8040  |
| 40               | 1260           | 2510 | 3770 | 5030 | 6280 | 7540 | 8800 | 10100 | 11300 | 12600 |

**Table 3.22** Cross-sectional area per metre width for various bar spacing (mm<sup>2</sup>)

| Bar size<br>(mm) | Spacing of bars |       |       |       |      |      |      |      |      |
|------------------|-----------------|-------|-------|-------|------|------|------|------|------|
|                  | 50              | 75    | 100   | 125   | 150  | 175  | 200  | 250  | 300  |
| 6                | 566             | 377   | 283   | 226   | 189  | 162  | 142  | 113  | 94.3 |
| 8                | 1010            | 671   | 503   | 402   | 335  | 287  | 252  | 201  | 168  |
| 10               | 1570            | 1050  | 785   | 628   | 523  | 449  | 393  | 314  | 262  |
| 12               | 2260            | 1510  | 1130  | 905   | 754  | 646  | 566  | 452  | 377  |
| 16               | 4020            | 2680  | 2010  | 1610  | 1340 | 1150 | 1010 | 804  | 670  |
| 20               | 6280            | 4190  | 3140  | 2510  | 2090 | 1800 | 1570 | 1260 | 1050 |
| 25               | 9820            | 6550  | 4910  | 3930  | 3270 | 2810 | 2450 | 1960 | 1640 |
| 32               | 16100           | 10700 | 8040  | 6430  | 5360 | 4600 | 4020 | 3220 | 2680 |
| 40               | 25100           | 16800 | 12600 | 10100 | 8380 | 7180 | 6280 | 5030 | 4190 |

**Table 3.13** Values of  $A_{sv}/s_v$

| Diameter<br>(mm) | Spacing of links (mm) |       |       |       |       |       |       |       |       |       |       |
|------------------|-----------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
|                  | 85                    | 90    | 100   | 125   | 150   | 175   | 200   | 225   | 250   | 275   | 300   |
| 8                | 1.183                 | 1.118 | 1.006 | 0.805 | 0.671 | 0.575 | 0.503 | 0.447 | 0.402 | 0.366 | 0.335 |
| 10               | 1.847                 | 1.744 | 1.57  | 1.256 | 1.047 | 0.897 | 0.785 | 0.698 | 0.628 | 0.571 | 0.523 |
| 12               | 2.659                 | 2.511 | 2.26  | 1.808 | 1.507 | 1.291 | 1.13  | 1.004 | 0.904 | 0.822 | 0.753 |
| 16               | 4.729                 | 4.467 | 4.02  | 3.216 | 2.68  | 2.297 | 2.01  | 1.787 | 1.608 | 1.462 | 1.34  |